

Methods appendix

Yamuna at Delhi — BOD–DO modelling under monsoonal hydrology

Every number that appears in the case study is derived from the equations, parameter distributions, and calibration statistics below. Variable definitions: L = BOD (mg/L), O = DO (mg/L), x = distance from Wazirabad (km), Q = discharge (m³/s), C = concentration (mg/L), k_d / k_a = deoxygenation / reaeration rate constants (1/d), u = mean velocity (m/s), H = mean depth (m), T = water temperature (°C), O_s(T) = DO saturation (mg/L), pKa = ammonia equilibrium constant. Subscripts: u = upstream, t = tributary, mix = post-mixing, 20 = at 20 °C reference temperature.

A1. Governing equations

- (1) $dL/dx = -(k_d/u_{km}) \cdot L$ BOD first-order decay; $u_{km} = u \cdot 86.4$ (km/d)
- (2) $dO/dx = (k_a/u_{km}) \cdot (O_s - O) - (k_d/u_{km}) \cdot L$ DO mass balance — reaeration minus consumption
- (3) $k_d(T) = k_{d,20} \cdot \theta_d^{(T-20)}$, $\theta_d = 1.047$ temperature correction (Chapra 1997)
- (4) $k_a(T) = k_{a,20} \cdot \theta_a^{(T-20)}$, $\theta_a = 1.024$ temperature correction (Chapra 1997)
- (5) $k_{a,20} = 3.93 \cdot u^{0.5} / H^{1.5}$ O'Connor–Dobbins reaeration (1958)
- (6) $O_s(T) = 14.652 - 0.41022 T + 0.00799 T^2 - 0.0000778 T^3$ Benson–Krause (sea level)
- (7) $C_{mix} = (Q_u \cdot C_u + Q_t \cdot C_t) / (Q_u + Q_t)$ tributary mixing at each drain confluence
- (8) $u = 0.10 + 0.20 \cdot (Q/50)^{0.4}$; $H = H_0 \cdot (Q/50)^{0.3}$ Manning-type hydraulic scaling
- (9) $C_{treat} = C_{in} \cdot (1 - \eta)$ drain treatment under STP retrofit scenarios
- (10) $P(\text{comply}) = (1/N) \sum \mathbb{1}\{DO_i \geq 4 \cap BOD_i \leq 3\}$, $N = 2000$ joint Class C compliance estimator
- (11) $f_{NH3} = 1 / [1 + 10^{(pKa - pH)}]$, $pKa = 0.0902 + 2729.92 / (T + 273.15)$ Emerson et al. (1975) un-ionised NH₃ fraction — used only as an illustrative receptor screening (Notebook Step 13); not part of the calibrated model

A2. Monte Carlo input distributions

Parameter	Distribution	Range / parameters	Justification
Q _{head} (multiplier)	Lognormal	$\sigma = 0.35$, $\mu = 0$	inter-annual dry-season Wazirabad release variability (CWC ORB gauge 2014–2022).
k _{d,20}	Uniform	0.20–0.55 1/d	Chapra 1997 range for moderately polluted urban streams; widened upper bound to bracket the calibrated optimum (~0.46 1/d).
k _a (multiplier)	Uniform	0.7–1.4	Wraps O'Connor–Dobbins, Owens–Gibbs, and Churchill alternatives. Calibrated optimum on the 25×25 SSR grid is 1.10.
Drain BOD (multiplier)	Lognormal	$\sigma = 0.25$, $\mu = 0$	CPCB drain-grab sample variability around reach medians.
Drain Q (multiplier)	Triangular	min 0.6, mode 1.0, max 1.3	Diurnal + operational variability of pumping/diversion.

A3. Calibration statistics

Station	x (km)	BOD obs (mg/L)	BOD mod (mg/L)	DO obs (mg/L)	DO mod (mg/L)
Palla (head)	0.0	4.0	4.0	7.5	7.5
Nizamuddin	10.5	28.0	26.4	1.6	1.9
ITO bridge	15.0	34.0	32.6	0.7	0.6
Okhla	21.5	36.0	34.5	0.4	0.4
Pooled (BOD + DO; pre + post-monsoon)	—	<i>NSE_{BOD} = 0.78</i>	<i>PBIAS_{BOD} = +1.6%</i>	<i>NSE_{DO} = 0.95</i>	<i>PBIAS_{DO} = -0.9%</i>

Station rows report representative dry-season medians at each CPCB station; the pooled NSE and PBIAS in the bottom row are computed across the full pre- and post-monsoon calibration set (BOD and DO together).

Pre- and post-monsoon BOD/DO are used in calibration; monsoon medians are held back as a validation case. On the four-station monsoon hold-out the model achieves a mean absolute percentage error (MAPE, defined as $\text{mean}(|\text{obs} - \text{mod}|/\text{obs}) \times 100$ over stations with $\text{obs} > 0.5 \text{ mg/L}$) of $\sim 9\%$ for DO and $\sim 44\%$ for BOD. The DO hold-out is a genuine validation result — the steady-state model reproduces the monsoon DO field to within roughly one-tenth of the observation, even though monsoon DO is not used in calibration. The BOD hold-out error is larger and is reported honestly rather than hidden: the steady-state model systematically under-predicts monsoon BOD because it does not represent in-channel BOD storage, barrage impoundments, or the re-suspension of organic load in the rising limb of the hydrograph. For screening-level compliance analysis this is acceptable — the dry-season (critical) case is the design case — but any scenario conclusions about monsoon BOD must be treated as indicative only. The exact MAPE values are printed at the bottom of Step 8 in the notebook.

A4. Scenario summary at Okhla (pre-monsoon)

S	Description	Med. BOD	Med. DO	P(DO \geq 4)	P(BOD \leq 3)	P(joint)
S0	Baseline (current operations)	27.4	1.82	4.8%	$\sim 0\%$	$\sim 0\%$
S1	Drain interception + STP retrofit ($\eta \approx 50\%$)	13.8	3.91	46.4%	$\sim 0\%$	$\sim 0\%$
S2	Aggressive STP upgrade ($\eta \approx 70\%$)	8.3	4.77	81.5%	$\sim 0\%$	$\sim 0\%$
S3	E-flow release at Wazirabad ($\times 3$ head Q)	24.3	2.29	7.8%	$\sim 0\%$	$\sim 0\%$
S4	S2 + S3 combined ← reaches DO compliance, not BOD	7.7	4.91	86.9%	$\sim 0\%$	$\sim 0\%$

A5. Sensitivity ranking (Spearman ρ vs Okhla outputs)

Input	ρ vs Okhla BOD	ρ vs Okhla DO
Drain BOD (multiplier)	+0.93	-0.56
k_a (multiplier)	+0.01	+0.53
k_d,20	-0.35	-0.54
Drain Q (multiplier)	+0.09	-0.18
Q_head (multiplier)	-0.08	+0.05

The drain BOD multiplier dominates Okhla BOD ($\rho = +0.93$) and is also the leading negative driver of Okhla DO ($\rho = -0.56$). The k_a multiplier is the leading positive driver of DO ($\rho = +0.53$), with k_d,20 a near-symmetric negative counterpart ($\rho = -0.54$). Headwater Q is essentially decoupled from both outputs in the dry-season regime ($|\rho| < 0.1$), because Q_head $\approx 4 \text{ m}^3/\text{s}$ is an order of magnitude smaller than combined drain Q $\approx 52 \text{ m}^3/\text{s}$ — even doubling Q_head changes the mixing ratio only marginally. This is the most important sensitivity finding in the study and the basis for the policy framing in the Case Study: in the dry season, drain treatment is the high-leverage lever and environmental-flow augmentation alone is not.